# ISLAMIC UNIVERSITY OF TECHNOLOGY (IUT) <br> ORGANISATION OF ISLAMIC COOPERATION (OIC) DEPARTMENT OF CIVIL AND ENVIRONMENTAL ENGINEERING 

Mid Semester Examination
Course No.: CEE 4543
Course Title: Foundation Engineering

Winter Semester: 2022-2023
Full Marks: 75
Time: 1.5 Hours

There are 4 (Four) questions. Answer 3 (Three) questions. Question 1 is compulsory. Answer 2 questions from questions 2, 3, and 4. Programmable calculators are not allowed. The figures in the right margin indicate full marks.
(a). Briefly explain the procedure of the Standard Penetration Test.

1(b). Derive the equation of virtual work per unit length ( $\left.W_{D}=c \cdot v_{i x}\right)$ in a discontinuous surface using Mohr-Coulomb failure criteria for $c-\phi$ soil.

I(c). Briefly explain a pile load test with a neat sketch of a schematic diagram of the test arrangement. Also, plot a typical load settlement curve for a pile load test.
2. A footing of $2.9 \mathrm{~m} \times 3.3 \mathrm{~m}$ is placed at a depth of 2.1 m from the ground surface. The vertical load of 2.2 MN , bending moments about the shorter direction of 480 $\mathrm{kN}-\mathrm{m}$, and bending moment about the longer direction of $360 \mathrm{kN}-\mathrm{m}$ are acting on the footing. The ground conditions are shown in Fig.1.
Answer the following:
(i) Estimate the ultimate bearing capacity of the footing. Use Meyerhof equations for bearing capacity factors, shape factors, and depth factors.
(ii) Estimate the effective allowable load capacity of the footing using both Meyerhof and the Bowles equation for eccentricity. Use a factor of safety of 2.5.
(iii) Calculate the maximum and minimum stresses acting on the soil for the design load and check with the strength of the soil, where the strength of the soil is 300 kPa .


Fig. 1
3(a). Compute the dimensions of a rectangular combined footing ( $B$ and $L$ ) for supporting
two columns (in column 1, $\mathrm{Q}_{1}=7.0 \mathrm{MN}$, in column 2, $\mathrm{Q}_{2}=5.0 \mathrm{MN}$ ) placed at a distance of 4.5 m . The second column is located 1.2 m from the property line. The

3(b). Compute the dimensions of a trapezoidal combined footing $\left(B_{1}, B_{2}\right.$, and $\left.L\right)$ for the

3(c). Size of a pile is $350 \mathrm{~mm} \times 350 \mathrm{~mm}$ having a length of 18 m , and the top of the pile is at ground level. The soil layers are as follows-

## Soil Layers:

$0-4 \mathrm{~m}$, sand, unit weight $=17.5 \mathrm{kN} / \mathrm{m}^{3}$, soil friction angle $-28^{0}, \delta=19^{\circ}$. $K=0.90$, and
$4-25 \mathrm{~m}$, sand, unit weight $=18.81 \mathrm{kN} / \mathrm{m}^{3}$, soil friction angle $=34^{\circ}, \delta=24^{0}$, $\mathrm{K}=1.4, \mathrm{~N}_{\mathrm{q}}=125$.
Use, critical Depth $(\mathrm{L} / \mathrm{D})_{\text {er }}=15 \mathrm{D}$
Estimate the ultimate axial load capacity of the pile considering water table is at 3 m below the ground level. Use, $\gamma_{v}=9.81 \mathrm{kN} / \mathrm{m}^{3}$.

4(a). Size of a pile is 16 inch $\times 16$ inch having a length of 60 feet, and the top of the pile is at ground level. The soil layers are as follows-

## Soil Layers:

$0-30 \mathrm{ft}$, normally consolidated clay, unit weight $=105 \mathrm{pcf}$,
Unconfined compression strength $=0.80 \mathrm{ksf}$, reduction factor $=0,90$
$30-120 \mathrm{ft}$, clay, unit weight $=125 \mathrm{pcf}$,
Unconfined compression strength $=2.0 \mathrm{ksf}$, reduction factor $=0.80$
Below 120 ft a thick deposit of dense sand exits.
Estimate the axial compression capacity of the pile considering water table is at 20 ft below the ground level. Use, $N_{\mathrm{e}}=9$, and $\gamma_{\mathrm{w}}=62.4$ pcf. Draw a neat sketch showing the stated condition.

4(b). Calculate the axial compression capacity of a 30 inch diameter and 50 feet length (top at ground level) drilled shaft. The ground condition is shown in Fig. 2 which consists of sandy soil.


Fig. 2

