# ISLAMIC UNIVERSITY OF TECHNOLOGY (IUT) <br> ORGANISATION OF ISLAMIC COOPERATION (IC) DEPARTMENT OF CIVIL AND ENVIRONMENTAL ENGINEERING 

Semester Final Examination
Course Number: CEE 4711
Course Title: Structural Analysis and Design II

Winter Semester: 2022-2023
Full Marks: 150
Time: 3 Hours

There are 7 (Seven) questions, Question no, 1 is COMPULSORY, Answer 6 questions including question 1. The figures in the right margin indicate full marks. COs and POs are also specified in the right margin of the questions. The symbols have their usual meaning.

1. (a) Derive the displacement and force transformation matrices for the frame (CO1) member shown in Fig. 1. The local coordinate systems are denoted by (PO1) ( $x^{\prime}, y^{\prime}, z^{\prime}$ ) and the global coordinate systems are denoted by ( $x, y, z$ ).


Fig. 1 for Question 1 (a)
(b) Draw the quantitative influence line (showing coordinates at $\mathrm{A}, \mathrm{B}, \mathrm{C}, \mathrm{D}$, (CO3) E, F, and G) for the support reactions, shear at C and moment at F for (PO2) the beam shown in Fig. 2. EI is constant.


Fig. 2 for Question 1(b)
2. Analyze the beam shown in Fig. 3 using stiffness method and determine (CO2) reactions at the supports and draw the shear force and bending moment diagram. (PO2) EI is constant. Horizontal movement of the beam is neglected.


Fig. 3 for Question 2
3. Analyze the frame shown in Fig. 4 using flexibility method and determine the (CO2)
reactions at the supports. El is constant.
(PO2)

Fig. 4 for Question 3

4. Analyze the beam shown in Fig. 5 using moment distribution method and (CO2)
(PO2) determine the support reactions and draw the shear force and bending moment (PO2) diagram. ET is constant.


Fig. 5 for Question 4
5. Analyze the beam shown in Fig. 6 using stiffness method and determine (CO2) reactions at the supports and draw the shear force and bending moment diagram. El is constant.


Fig, 6 for Question 5
6. Analyze the frame shown in Fig. 7 using moment distribution method and (CO2) determine the support reactions and end moment of all the members. EI is
(PO2) constant


Fig. 7 for Question 6
7. Analyze the beam shown in Fig. 8 using flexibility method and determine the reactions at the supports and draw the shear foree and bending moment diagram.
(PO2)


Fig. 8 for Question 7
$\left[\begin{array}{l}q_{N v} \\ q_{N z} \\ q_{V y} \\ q_{V z}\end{array}\right]=\left[\begin{array}{rrrr}N_{y} & N_{z} & F_{Y} & F_{z} \\ \frac{12 E I}{L^{1}} & \frac{6 E I}{L^{2}} & -\frac{12 E I}{L^{3}} & \frac{6 E I}{L^{2}} \\ \frac{6 E I}{L^{2}} & \frac{4 E I}{L} & -\frac{6 E I}{L^{2}} & \frac{2 E I}{L} \\ -\frac{12 E I}{L^{3}} & -\frac{6 E I}{L^{2}} & \frac{12 E I}{L^{3}} & -\frac{6 E I}{L^{2}} \\ \frac{6 E I}{L^{2}} & \frac{2 E I}{L} & -\frac{6 E I}{L^{2}} & \frac{4 E I}{L}\end{array}\right]\left[\begin{array}{l}d_{N y} \\ d_{N z z} \\ d_{F \gamma} \\ d_{F z}\end{array}\right]$


Fixed End Moments


Table for Evaluating $\int_{0}^{L} m m^{\prime} d x$

| $\int_{1}^{l} m m^{\prime} d r$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $m m^{\prime} L$ | $\frac{1}{2} m m^{\prime} / 2$ | $\frac{1}{2} m(m i+m i l e$ | $\frac{2}{3} \operatorname{man}^{2}$ ' |
|  | $\frac{1}{2} m m / 2$ | $\frac{1}{3} m m / 2$ | $\frac{1}{6} m(m i+2 m \mid x 2$ | $\frac{5}{12} \mathrm{~mm} \mathrm{~m}^{2}$ |
|  | $\frac{1}{2} m^{\prime}\left(m_{1}+m_{2}\right) l$ | ${ }_{6}^{1} m^{2}\left(m_{5}+2 m\right) t$, | $\begin{aligned} & \frac{1}{6}\left[m_{i}\left[2 m_{1}+m_{p}\right\rangle\right. \\ & +m\left\{\left[m_{1}+2 m_{1}\right]\right] L \end{aligned}$ | $\frac{1}{12}\left[m^{\prime}\left(3 m_{1}+5 m_{2}\right]\right.$ ] |
|  | $\frac{1}{2} \min 2$ | $\frac{1}{6} \operatorname{man}^{\prime}(L+\Delta)$ | $\frac{1}{6} m_{\left.m_{0}(L)(L+a)\right]}$ | $\frac{1}{12} m m^{\prime}\left(3+\frac{3 n}{L}-\frac{L^{2}}{L^{2}}\right) L$ |
| m | $\frac{1}{2} m m^{\prime} / 2$ | $\frac{1}{6} m m^{\prime} L$ | $\frac{1}{4} m(2 m i+m) 2$ | $\frac{1}{4} m m \cdot 2$ |

## Beam Deflections and Slopes

| Lauding | $\ldots \uparrow$ | $0+\pi$ | $\text { Equation }+\uparrow+\Gamma$ |
| :---: | :---: | :---: | :---: |
|  | $\mathrm{s}_{\text {asi }}=\frac{P L^{\prime}}{3 E 1}$ | $\begin{aligned} & \mathrm{a}_{2 x}-\frac{P_{I}^{2}}{2 F I} \\ & x A=L \end{aligned}$ | $=-\frac{p}{6 E I}\left(x^{x}-34 x^{2}\right)$ |
|  | $\begin{aligned} & 4=-\frac{M_{0} L^{2}}{2 E T} \\ & 2 A T=t \end{aligned}$ | $\begin{aligned} & \tilde{n}_{\text {en }}=\frac{N_{0} l}{E I} \\ & \text { at } \pi-L \end{aligned}$ | $v=\frac{M_{o}}{2 \hbar I} r^{2}$ |

Beam Deflections and Slopes (continued)

|  | $\begin{aligned} & V_{\text {tex }}=\frac{w L^{4}}{8 E I} \\ & a 1 x=L \end{aligned}$ | $\begin{aligned} & \sigma_{\text {mat }}=\frac{n L^{2}}{6 E t} \\ & a t N=L \end{aligned}$ | $v=\frac{v}{24 E 1}\left(x^{t}-4 L^{2}+4 C^{2} x^{2}\right)$ |
| :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & V_{10 t}=\frac{P L}{4 P E!} \\ & \text { at } 1=L / 2 \end{aligned}$ |  | $\begin{aligned} & v=\frac{P}{4 X E I}\left(4 x^{2}-3 L^{2} x\right) . \\ & U \leqslant x \leq L / 2 \end{aligned}$ |
|  |  | $\begin{aligned} & v_{L}=\frac{P_{\text {abc } L}(+b)}{6 L E T} \\ & d=\frac{P_{\text {ah }}(L+n)}{6 L E I} \end{aligned}$ | $\begin{aligned} & N=\frac{P(E x}{6 L E J}\left(L^{T}-N-A\right) \\ & N=x \approx v \end{aligned}$ |
|  |  | $t_{\text {a }}=-\frac{w L^{2}}{24 E 1}$ | $v=\frac{k t}{2 t k t}\left(x^{2}-2 t s^{2}+2\right)^{2}$ |
|  |  |  | $\begin{aligned} & =-\frac{w x}{384 E I}\left(26 x^{2}-24 L x^{2}+9 x^{2}\right) \\ & u \leqslant x \leqslant L / 2 \\ & \left.=-\frac{w L}{384 E y^{2}}\left(8 x^{2}-24 L x^{2}+17 L^{2} x-L\right)^{2}\right) \\ & L / 2 \leqslant x \leq L \end{aligned}$ |
|  | $M_{\text {men }}=\frac{M_{0} L^{5}}{\sqrt{3 E I}}$ | $\begin{aligned} & J_{C}=\frac{M_{0} L}{\text { GEI }} \\ & a_{n}=\frac{M_{0} L}{3 E I} \end{aligned}$ | $x-\frac{M_{D a}}{6 E I L}\left(L^{t}-x^{t}\right)$ |

